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## 10 11 **Discussion of “CPT evaluation of yield stress profiles in soils”**

12 **by Shehab S. Agaiby and Paul W. Mayne**

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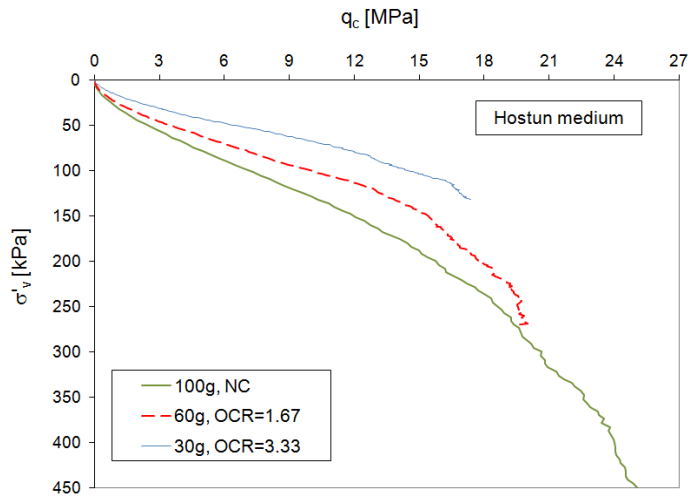
18 The Authors presented a comprehensive approach concerning the estimation of yield stress  
19 profile in different soils. They considered a large database including worldwide well-  
20 documented test sites and calibration chamber tests. I would like to focus the discussion on the  
21 evaluation of OCR ratio in sands in the continuous mini-cone penetration using the results of  
22 centrifuge tests. Physical modelling in centrifuge permits to perform some parametric studies  
23 including the effect of soil overconsolidation. The aim of this discussion is to verify the  
24 correlations for OCR in sands - proposed by the Authors - in case of defined uniform OCR  
25 within the soil profile. A series of centrifuge CPT tests (Bałachowski, 1995) was conducted in  
26 dense  $D_R=0.82$  uniform quartz sand using mini-CPT model ( $B=12$  mm). The soil mass with  
27 medium Hostun sand ( $d_{50}=0.32$  mm) was prepared using sand raining technique. The  
28 overconsolidated soil mass was obtained by reducing the centrifuge g-level from 100g to 60g  
29 or 30g (with overconsolidation ratio,  $OCR=1.67$  or  $3.33$ , respectively). In this way a uniform  
30 OCR ratio was achieved in the soil profile. The mini-cone penetration tests were conducted in

31 normally consolidated sand, i.e. at 100g and in overconsolidated sand after reduction of  
32 centrifuge acceleration to 60g (OCR=1.67) and then to 30g (OCR=3.33).

33 The results of cone resistance in Hostun medium sand are shown in Fig. 1. One can notice that  
34 at a given vertical stress higher cone resistance is mobilized in overconsolidated soil mass. In  
35 case of shallow penetration scheme the normalized cone resistance increases almost linearly  
36 with vertical stress (Fig. 2). It can be also noticed that the critical depth increases with OCR  
37 ratio. Calibration chamber tests (Jamiolkowski et al. 1985) have shown that under critical depth  
38 the cone resistance is proportional to the square root of vertical stress. It was also confirmed in  
39 cone penetration tests in centrifuge (Gui et al. 1998, Bolton et al. 1999, Bałachowski, 2007,  
40 Salgado, 2014 and Kim et al. 2015).

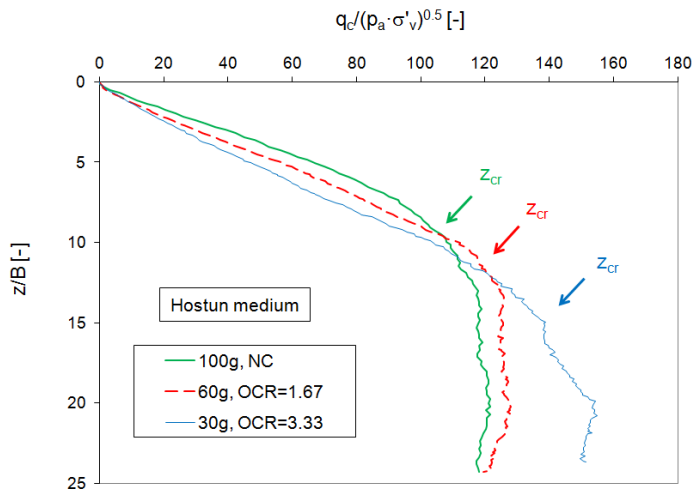
41 The OCR ratio for cone penetration tests (Fig. 3) was calculated using the Eq. 7 proposed by  
42 the Authors. In case of shallow penetration mode the calculated OCR values increase with  
43 vertical stress and reach the maximum value near the critical depth, and then attenuate with  
44 further penetration. The calculated OCR is closer to the simulated value at large penetration  
45 depths (vertical stress). It is interesting to note that for normally consolidated sand the  
46 calculated OCR values are higher than one and similar shape of calculated OCR profile is  
47 observed in normally consolidated and overconsolidated sands. The general observation is that  
48 the proposed formula for yield stress (Eq. 7) can be used only in case of deep penetration  
49 scheme and seems to overestimate OCR ratio in sands. If the grain size effects is rather  
50 negligible (Gui et al. 1998) in the considered tests ( $B/d_{50}=37.5$ ) the geometrical size effects  
51 could however influence the results at lower stress level. Further studies including the procedure  
52 “modelling of models” will be necessary to study this effect.





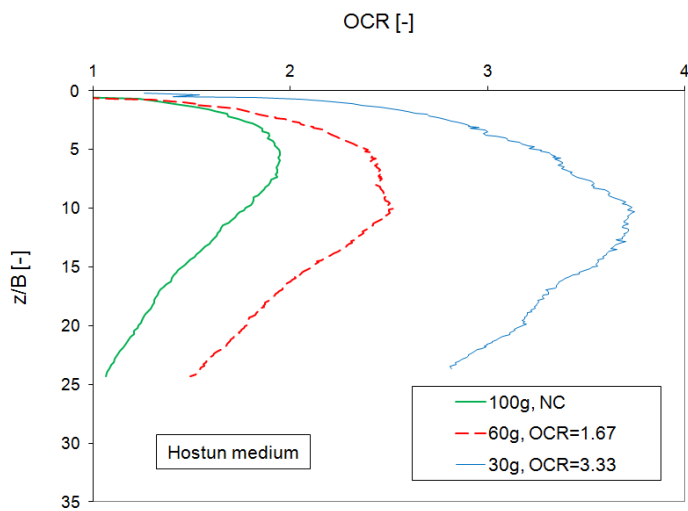
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54 Fig. 1. Mini-cone penetration tests in Hostun medium sand.



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56 Fig. 2. Normalized cone resistance vs. normalized penetration depth.



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58 Fig. 3. OCR ratio derived from mini-cone penetration tests.

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