

Advanced Scalar-valued Intensity Measures for Residual Drift Prediction of SMRFs with Fluid Viscous Dampers

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Abstract—Maximum Residual Inter-story Drift Ratio ($RIDR_{max}$) plays an important role to specify the state of a structure after severe earthquake and the possibility of repairing the structure. Therefore, it is necessary to predict the $RIDR_{max}$ of Steel Moment-Resisting Frames (SMRFs) with high reliability by employing powerful Intensity Measures (IMs). This study investigates the efficiency and sufficiency of scalar-valued IMs for predicting $RIDR_{max}$ of two sets of the 3-Story, 6-Story, and 9-Story SMRFs with and without using linear Fluid Viscous Dampers (FVDs). Incremental Dynamic Analysis (IDA) was performed with considering $RIDR_{max}$ as engineering demand parameters using Opensees. Results of analysis showed that two scalar-valued IMs of $IM_M(\alpha=0.5)$ and $S_{avg\ M-D}$ had lower values of the variations of standard deviation of natural logarithm of IM of $RIDR_{max}$, $\sigma \ln IM_{RD}$, which shows the efficiency of these IMs. Moreover, these scalar-valued IMs achieved higher p -values with respect to seismic ground motion features of M , R , and $Vs30$, which shows the sufficiency of assumed IMs. Therefore, two scalar-valued IMs of $IM_M(\alpha=0.5)$ and $S_{avg\ M-D}$ are proposed as optimal scalar-valued IMs for predicting the $RIDR_{max}$ of SMRFs.

Keywords—scalar-valued intensity measure, residual drift, spectral shape, fluid viscous damper, incremental dynamic analysis

I. INTRODUCTION

Reliable design of structures against natural hazards such as earthquakes requires fully understanding the influence of seismic parameters on the structural behavior [1-5]. Hence, it is vital to identify ground motion properties, which are referred to as Intensity Measures (IMs), to assess the seismic response of buildings. The IMs are used to quantify the severity of a seismic event and uncertainty of them using one parameter or a vector of a few parameters. These parameters are related to a set of well-selected ground motion records. Efficiency, which is the most important characteristic of an IM, means good explanatory power of the IM regarding

Engineering Demand Parameter (EDP) to reduce the number of records of analysis under given accuracy. While sufficiency means the ability of the IM for predicting the response of structure independent from other record properties. Therefore, an appropriate IM should satisfy the properties of efficiency and sufficiency [6-8]. In general, IMs are divided into two groups of scalar-valued and vector-valued IMs according to the dimension of parameters (e.g., see [9, 10]). Scalar-valued IMs imply the relationship between IM and EDP in a two-dimensional coordinate system using one parameter. Within the past years, numerous research studies comprehensively investigated the scalar-valued IMs, the peak ground acceleration (PGA), and elastic spectral acceleration (S_a) at the fundamental period of a structure, denoted as T_1 , were introduced as the most-used scalar IMs [11-14]. Recent seismic events have demonstrated that some damaged buildings may need to be demolished due to excessive permanent lateral deformations at the end of the earthquake, even without suffering total collapse or severe damages. Therefore, maximum Residual Inter-story Drift Ratio ($RIDR_{max}$) at all story levels or roof play a crucial role in defining the seismic performance of the structure, determining the feasibility of retrofitting damaged structure as well as estimating the structural residual capacity [15, 16]. Therefore, the main aim of this study is to investigate the performance of scalar-valued IMs to calculate $RIDR_{max}$ of Steel Moment-Resisting Frames (SMRFs) with and without using Fluid Viscous Dampers (FVDs). Thereby, this study proposes three “optimal” scalar-valued IMs based on the effects of spectral shape and ground motion duration for predicting $RIDR_{max}$ of the framed steel structures within a certain confidence level.

II. SCALAR-VALUED INTENSITY MEASURE

The scalar-valued IMs can be categorized into two groups as structure-specific IMs and non-structure-specific IMs including 12 and 13 IMs, respectively (see Tables I and II [7]). In this study, structure-specific IMs are further categorized into three groups as spectral,

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spectral shape-based and combined spectral shape-based and duration-based. Similarly, non-structure-specific IMs are also classified into three sets as acceleration-related (including five IMs), velocity-related (including five IMs) and displacement-related (including three IMs). It is worth noting here that structure-specific IMs, employed in this study, are calculated from the response of the spectral components of a ground motion record assuming 5% damping value. Whereas, non-structure-specific IMs are obtained from the ground-motion time histories (further information can be found in [7]). Based on the literature review, Tables I and II present scalar-valued IMs that were used in this study.

TABLE I. NON-STRUCTURE-SPECIFIC SCALAR-VALUED IMs

Notation	Name	Definition
<i>Acceleration-related scalar-valued IMs</i>		
PGA	Peak Ground Acceleration	$PGA = \max a(t) $
AI	Arias Intensity	$AI = \frac{\pi}{2g} \int_0^{t_f} a(t)^2 dt, t_f = \text{total duration}$
Ic	Characteristic Intensity	$IC = (a_{rms})^{1.5} \cdot t_d^{0.5}, a_{rms} = \sqrt{\frac{1}{t_d} \int_{t_1}^{t_2} a(t)^2 dt}, t_d = t_2 - t_1$
Ia	Compound Acceleration IM	$IA = PGA \cdot t_d^{1/3}$
CAV	Cumulative Absolute Velocity	$CAV = \int_0^{t_f} a(t) dt$
<i>Velocity-related scalar-valued IMs</i>		
PGV	Peak Ground Velocity	$PGV = \max v(t) $
FI	Fajfar Intensity	$FI = PGV \cdot t_d^{0.25}$
Iv	Compound Velocity IM	$I_v = PGV^{2/3} \cdot t_d^{1/3}$
CAD	Cumulative Absolute Displacement	$CAD = \int_0^{t_f} v(t) dt$
SED	Specific Energy Density	$SED = \int_0^{t_f} v(t)^2 dt$
<i>Displacement-related scalar-valued IMs</i>		
PGD	Peak Ground Displacement	$PGD = \max d(t) $
Id	Compound Displacement IM	$I_d = PGD \cdot t_d^{1/3}$
CAI	Cumulative Absolute Impulse	$CAI = \int_0^{t_f} d(t) dt$

TABLE II. STRUCTURE-SPECIFIC SCALAR-VALUED IMs

Notation	Name	Definition
<i>Spectral and spectral-shaped scalar-valued IMs</i>		
$S_a(T_1)$	Spectral Acceleration at T_1	
ASI	Acceleration Spectrum Intensity	$ASI = \int_{0.1}^{0.5} S_a(T, 5\%) dt$
SI	Spectrum Intensity	$SI = \int_{0.1}^{2.5} S_v(T, 5\%) dt$
DSI	Displacement Spectrum Intensity	$DSI = \int_2^5 S_d(T, 5\%) dt$
IMc		$IM_c = S_a(T_1) \cdot (S_a(T_2) / S_a(T_1))^{0.5}, T_2 = 2T_1$
IMM		$IM_M = S_a(T_1) \cdot (S_a(T_2) / S_a(T_1))^{0.5}, T_2 = R^\alpha T_1, \alpha = 0.5 \text{ or } 0.3$
INP		$I_{NP} = S_a(T_1) \cdot NP^{0.4}, NP = S_{avg}(T_1 \dots T_N) / S_a(T_1)$

$$S_{a,avg} = S_{a,avg}(C T_1 \dots C_N T_1) = \left(\prod_{i=1}^N S_a(C_i T_i) \right)^{1/N}$$

$$C_1 = 0.2, C_N = 3$$

Combined spectral duration and shape scalar-valued IMs

$$IM_{M-D} = S_a(T_1) \cdot (S_a(R^\alpha T_1) / S_a(T_1))^m \cdot t_d^\beta$$

$$I_{NPM-D} = S_a(T_1) \cdot (S_{avg}(T_1 \dots T_N) / S_a(T_1))^n \cdot t_d^\beta$$

$$T_N = R^\alpha T_1$$

$$S_{a,avg M-D} = S_{a,avg}(C T_1 \dots C_N T_1) \cdot t_d^\beta, C_N = R^\alpha$$

III. STRUCTURAL MODELING

In order to investigate efficiency and sufficiency of assumed scalar-valued IMs, two sets of SMRFs were considered. The first set includes the 3-, 6-, and 9-Story-SAC SMRFs that were designed for SAC project [17] and the detail of designing process can be found in FEMA 355C [18]. Fig. 1 depicts the elevation, section and arrangement of the FVDs on 3-, 6- and 9-Story-SAC SMRFs. The second set includes the 3-, 6-, and 9-Story-Ref SMRFs, designed by Kazemi et al. [19, 20] according to ASCE 7-10 [21]. Fig. 2 depicts the elevation, section and arrangement of the FVDs on the 3-, 6- and 9-Story-Ref SMRFs. The FVDs were placed diagonally within SMRFs to improve seismic performance of these structures. During last decade, many researchers focused on the importance of P-Delta effect, and some approaches were used to considering the P-Delta effect [see e.g. 1, 13, 22-25].

In this study, it was assumed all columns except those in the SMRFs behaves like as a leaning column to consider the P-Delta effects. In addition, to improve accuracy of modeling in OpenSees [26], deteriorating moment-rotation hysteresis according to the Modified Ibarra–Krawinkler bilinear-hysteretic model was used for beams and columns of the SMRFs. To find out the efficiency and sufficiency of proposed scalar-valued IMs, the improved SMRFs using linear FVDs were considered.

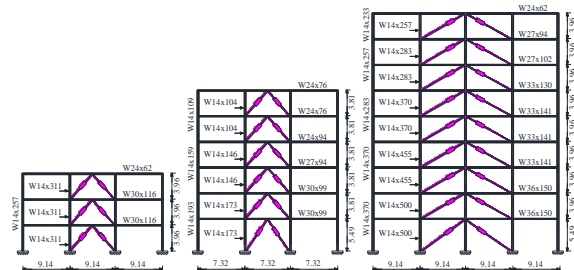


Figure 1. Dimensions and configuration of the linear FVDs on the 3-, 6- and 9-Story-SAC SMRFs.

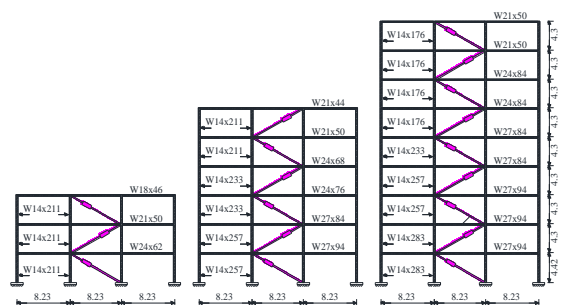


Figure 2. Dimensions and configuration of the linear FVDs on the 3-, 6- and 9-Story-Ref SMRFs.

To model linear FVDs, a uniform vertical distribution of damping coefficients were generated assuming a five percent Rayleigh damping ratio for the first and third modes of structures and supplemental viscous damping ratio of 0.15 ($\zeta_D=0.15$). The supplemental viscous damping ratio can be calculated as follows [2, 19, 27]:

$$\xi_D = \frac{\sum_{i=1}^{N_D} (\pi) \lambda_i C_i T_1 \cos \theta_i^2 \varphi_{n1}^2}{8\pi^3 \cdot \sum_{j=1}^{N_s} m_j \cdot \varphi_{j1}^2} \quad (1)$$

$$C_D = \frac{\xi_D \cdot 8\pi^3 \cdot \sum_{j=1}^{N_s} m_j \cdot \varphi_{j1}^2}{\sum_{i=1}^{N_D} \pi \lambda_i T_1 \cos \theta_i^2 \varphi_{n1}^2} \quad (2)$$

where the number of the FVD devices, N_D , the damping coefficient, C_i , the fundamental period of vibration, T_1 , the number of stories, N_s , the mass of story, m_j , the angle of damper direction, θ_i , can be defined based on the characteristics of designed SMRFs. In addition, other parameters like as the relative deformation between the horizontal degrees of freedom at the ends of the FVDs, φ_{n1} , the first mode component at the top of the story, φ_{j1} , can be achieved from the models. In order to use a uniform vertical distribution of damping coefficients, the Equation (2) was rearranged to calculate the damping coefficient for all linear FVDs, C_D . To perform Incremental Dynamic Analysis (IDA), ground motion records considered by Jamshidiha et al. [7, 8] were used. In addition, four $RIDR_{max}$ of 0.2%, 0.5%, 1.0%, and 2.0% were assumed according to Yahyazadeh et al. [28].

IV. INVESTIGATING THE EFFICIENCY OF THE IMs FOR RESIDUAL DRIFT PREDICTION

Achieving the $RIDR_{max}$ is essential for vulnerability assessment of a structure after a severe earthquakes, which shows the state of structure and the possibility of retrofiting or repairing of the structure. Therefore, it is essential to use a powerful IM with high reliability that makes the results more realistic. Efficiency is called the power of an IM to predict the seismic response (e.g. the $RIDR_{max}$ of SMRFs) with low dispersion. In this section, the efficiency of 25 structure-specific and non-structure-specific scalar-valued IMs presented in Tables I and II were investigated. Fig. 3 presents the variations of standard deviation of natural logarithm of IM of $RIDR_{max}$, $\sigma \ln IM_{RD}$, values for scalar-valued IMs in the 3-Story-SAC, 6-Story-SAC, and 9-Story-SAC SMRFs. It can be seen that for predicting the $RIDR_{max}$ of 0.002 in the 3-Story-SAC, 6-Story-SAC, and 9-Story-SAC SMRFs, the $\sigma \ln IM_{RD}$ values for $IM_M(\alpha=0.5)$ are equal to 0.25, 0.28, and 0.20, respectively, which is lower than other assumed scalar-valued IMs. For predicting the $RIDR_{max}$ of 0.005, 0.01, and 0.02 in the 3-Story-SAC SMRF, the $\sigma \ln IM_{RD}$ values for $Sa_{avg M-D}$ are equal to 0.31, 0.29, and 0.20, respectively. In addition, the $\sigma \ln IM_{RD}$ values for $Sa_{avg M-D}$ in the 6-Story-SAC SMRF are equal to 0.30, 0.25, and 0.22, respectively, and for the 9-Story-SAC SMRF are equal to 0.22, 0.23, and 0.20, respectively. Therefore, two scalar-valued IMs of $IM_M(\alpha=0.5)$ and $Sa_{avg M-D}$ have

lower $\sigma \ln IM_{RD}$ that shows the efficiency of assumed IMs. Moreover, these IMs have lower $\sigma \ln IM_{RD}$ values in the 3-Story-SAC, 6-Story-SAC, and 9-Story-SAC SMRFs with linear FVDs and can be considered as efficient IMs.

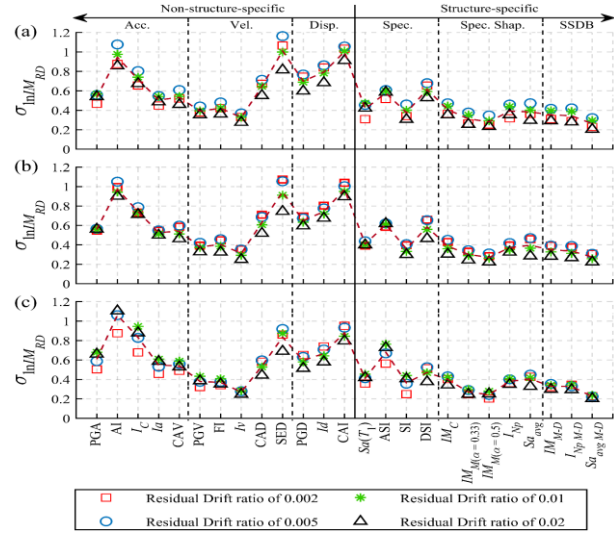


Figure 3. Variations of $\sigma \ln IM_{RD}$ values for scalar-valued IMs in the, a) 3-Story-SAC, b) 6-Story-SAC, and c) 9-Story-SAC SMRFs.

Fig. 4 presents the variations of $\sigma \ln IM_{RD}$ values for scalar-valued IMs in the 3-Story-Ref, 6-Story-Ref, and 9-Story-Ref SMRFs.

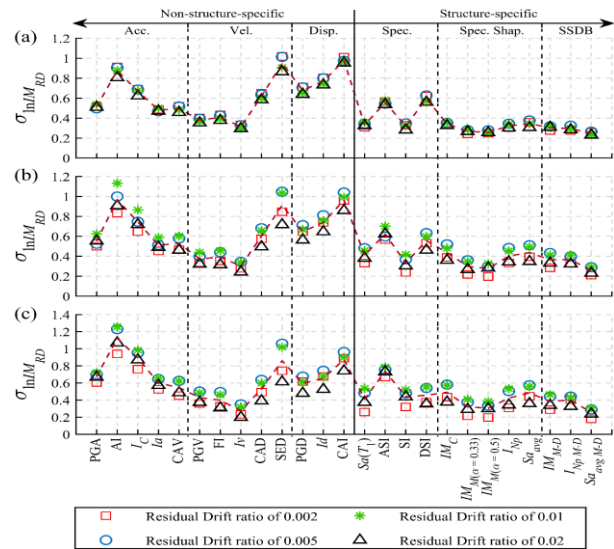


Figure 4. Variations of $\sigma \ln IM_{RD}$ values for scalar-valued IMs in the, a) 3-Story-Ref, b) 6-Story-Ref, and c) 9-Story-Ref SMRFs.

It can be seen that for predicting the $RIDR_{max}$ of 0.002 in the 3-Story-Ref, 6-Story-Ref, and 9-Story-Ref SMRFs, the $\sigma \ln IM_{RD}$ values for $IM_M(\alpha=0.5)$ are equal to 0.24, 0.19, and 0.18, respectively, which is lower than other assumed scalar-valued IMs.

For predicting the $RIDR_{max}$ of 0.005, 0.01, and 0.02 in the 3-Story-Ref SMRF, the $\sigma \ln IM_{RD}$ values for $Sa_{avg M-D}$ are equal to 0.26, 0.23, and 0.23, respectively. In addition, the $\sigma \ln IM_{RD}$ values for $Sa_{avg M-D}$ in the 6-Story-Ref SMRF are equal to 0.29, 0.29, and 0.23, respectively, and for the 9-Story-Ref SMRF are equal to 0.29, 0.30, and 0.23, respectively. In addition, IMs of $IM_M(\alpha=0.5)$ and $Sa_{avg M-D}$ have

had lower $\sigma \ln IM_{RD}$ values assuming linear FVDs. Therefore, these two IMs can be considered as efficient scalar-valued IMs for predicting $RIDR_{max}$ of SMRFs with and without linear FVDs. Table III presents the Fractional Reduction (FR) in the mean dispersion, $(\sigma \ln IM_{RD})_{avg}$, in four optimal scalar-valued IMs. It can be noted that the higher values of FR achieved for two IMs of $IM_M(\alpha=0.5)$ and $Sa_{avg M-D}$.

TABLE III. FRACTIONAL REDUCTION (FR) IN $(\sigma \ln IM_{RD})_{avg}$ ACHIEVED BY THE PROPOSED SCALAR-VALUED IMs WITH AND WITHOUT LINEAR FVDs.

		I_v		$IM_{M(\alpha=0.33)}$		$IM_{M(\alpha=0.5)}$		$Sa_{avg M-D}$	
		$(\sigma \ln IM_{RD})_{avg}$	FR (%)	$(\sigma \ln IM_{RD})_{avg}$	FR (%)	$(\sigma \ln IM_{RD})_{avg}$	FR (%)	$(\sigma \ln IM_{RD})_{avg}$	FR (%)
Without FVD	RD=0.2%	0.30	7.98	0.25	22.58	0.23	29.43	0.24	27.89
	RD=0.5%	0.34	22.59	0.34	22.40	0.30	30.69	0.28	35.03
	RD=1%	0.31	30.28	0.33	26.05	0.30	32.93	0.27	39.29
	RD=2%	0.25	35.01	0.26	32.27	0.26	33.52	0.22	42.72
With FVD	RD=0.2%	0.30	14.33	0.27	22.57	0.24	31.74	0.24	31.81
	RD=0.5%	0.31	27.67	0.33	24.14	0.29	32.25	0.28	34.89
	RD=1%	0.32	29.39	0.35	22.80	0.29	36.51	0.30	34.13
	RD=2%	0.27	36.70	0.30	32.01	0.27	36.63	0.25	42.39

V. INVESTIGATING THE SUFFICIENCY OF THE IMs FOR RESIDUAL DRIFT PREDICTION

Sufficiency is called the ability of an IM to render the seismic response (e.g. the $RIDR_{max}$ of SMRFs) independent from the other characteristics of the seismic ground motion records. Therefore, a sufficient IM prevents a biased distribution for the seismic response (e.g. the $RIDR_{max}$ of SMRFs) assessed from IDAs. To compare the sufficiency of the IMs, the p -value was calculated regarding the ground motion characteristics of M, R, and Vs30. Table IV presents the percent of structures with p -values ≥ 0.05 obtained from investigating the sufficiency of proposed scalar-valued IMs with respect to M, R, and Vs30.

TABLE IV. PERCENT OF STRUCTURES WITH p -VALUES ≥ 0.05 OBTAINED FROM INVESTIGATING THE SUFFICIENCY OF PROPOSED SCALAR-VALUED IMs WITH RESPECT TO M, R, AND Vs30.

	Scalar-valued IM	% of structures with p -values ≥ 0.05		
		M	R	Vs30
Without FVD	I_v	25	100	87.5
	$IM_{M(\alpha=0.33)}$	91.67	91.67	91.52
	$IM_{M(\alpha=0.5)}$	87.5	100	95.83
	$Sa_{avg M-D}$	95.83	100	91.69
With FVD	I_v	54.17	100	87.5
	$IM_{M(\alpha=0.33)}$	91.67	100	87.5
	$IM_{M(\alpha=0.5)}$	95.83	95.83	83.33
	$Sa_{avg M-D}$	79.17	100	91.67

The results show that two IMs of $IM_M(\alpha=0.5)$ and $Sa_{avg M-D}$ are selected as optimal scalar-valued IMs for predicting the $RIDR_{max}$ of considered SMRFs. Figs. 5 and 6 present the sufficiency of proposed scalar-valued IMs in the assumed $RIDR_{max}$ and for aforementioned SMRFs

with respect to seismic characteristics of M, R, and Vs30, without and with assuming linear FVDs, respectively.

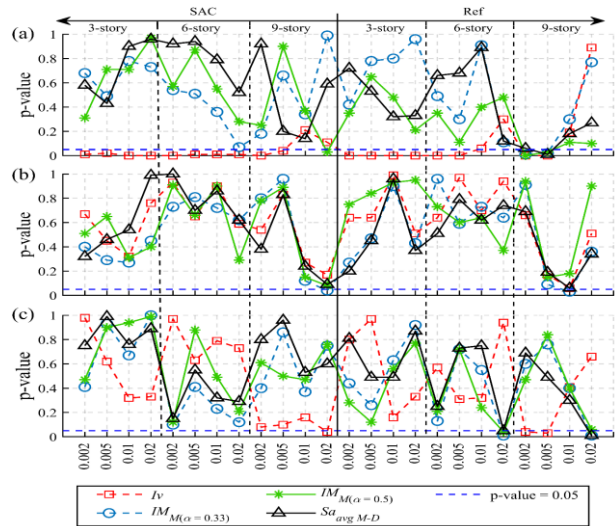


Figure 5. Sufficiency of proposed scalar-valued IMs in the assumed $RIDR_{max}$ and SMRFs, a) M, b) R, c) Vs30.

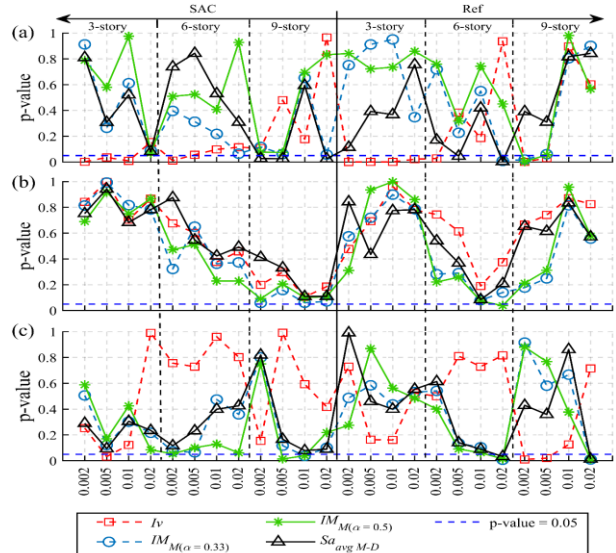


Figure 6. Sufficiency of proposed scalar-valued IMs in the assumed $RIDR_{max}$ and SMRFs with linear FVDs a) M, b) R, c) Vs30.

It can be seen that two scalar-valued IMs of I_v and $IM_M(\alpha=0.33)$ in some cases achieved p -values lower than 0.05 (5%) that shows the insufficiency of these IMs. While two IMs of $IM_M(\alpha=0.5)$ and $Sa_{avg M-D}$ had p -values higher than 0.05 in all cases. Therefore, these IMs are considered as sufficient IMs with respect to seismic characteristics of M, R, and Vs30.

VI. CONCLUSIONS

In this study, the efficiency and sufficiency of 25 scalar-valued IMs including non-structure-specific IMs and structure-specific IMs were selected to predict the $RIDR_{max}$ of two sets of the 3-Story, 6-Story, and 9-Story SMRFs with and without considering linear FVDs. The results of testing the efficiency of two scalar-valued IMs

of $IM_M(\alpha=0.5)$ and $Sa_{avg\ M-D}$ showed that they had lower values of $\sigma \ln IM_{RD}$ compared to other assumed scalar-valued IMs. In addition, two IMs of $IM_M(\alpha=0.5)$ and $Sa_{avg\ M-D}$ achieved higher values of FR in the mean dispersion, $(\sigma \ln IM_{RD})_{avg}$. Moreover, the sufficiency of them, which is another important factor, was compared. The p -value of two IMs of $IM_M(\alpha=0.5)$ and $Sa_{avg\ M-D}$ with respect to seismic ground motion features of M, R, and Vs30, were higher than 0.05, which shows the sufficiency of assumed IMs. Therefore, two scalar-valued IMs of $IM_M(\alpha=0.5)$ and $Sa_{avg\ M-D}$ are proposed as optimal IMs for predicting the $RIDR_{max}$ of SMRFs.

CONFLICT OF INTEREST

The authors declare no conflict of interest.

AUTHOR CONTRIBUTIONS

Farzin Kazemi and Neda Asgarkhani conducted the research and analyzed the data, Atefeh Yousefi wrote the paper; Benyamin Mohebi checked the paper; all authors had approved the final version.

REFERENCES

- [1] F. Kazemi, B. Mohebi, and M. Yakhchalian, "Evaluation the P-delta effect on collapse capacity of adjacent structures subjected to far-field ground motions," *Civil Engineering Journal*, vol. 4, no. 5, p. 1066, 2018, doi:10.28991/cej-0309156.
- [2] F. Kazemi, B. Mohebi, and M. Yakhchalian, "Enhancing the seismic performance of adjacent pounding structures using viscous dampers," in *Proc. the 16th European Conference on Earthquake Engineering (16ECEE)*, 18-21, June, Thessaloniki, Greece, 2018.
- [3] B. Mohebi, N. Asadi, and F. Kazemi, "Effects of using gusset plate stiffeners on the seismic performance of concentrically braced frame," *International Journal of Civil and Environmental Engineering*, vol. 13, no. 12, pp. 723-729, 2019.
- [4] B. Mohebi, O. Yazdanpanah, F. Kazemi, and A. Formisano, "Seismic damage diagnosis in adjacent steel and RC MRFs considering pounding effects through improved wavelet-based damage-sensitive feature," *Journal of Building Engineering*, vol. 33, 101847, 2021.
- [5] O. Yazdanpanah, B. Mohebi, F. Kazemi, I. Mansouri, R. Jankowski, "Development of fragility curves in adjacent steel moment-resisting frames considering pounding effects through improved wavelet-based refined damage-sensitive feature," *Mechanical Systems and Signal Processing*, vol. 173, 109038, 2022.
- [6] A. K. Kazantzi and D. Vamvatsikos, "Intensity measure selection for vulnerability studies of building classes," *Earthquake Engineering & Structural Dynamics*, vol. 44, no. 15, pp. 2677-2694, 2015.
- [7] H. R. Jamshidiha, M. Yakhchalian, and B. Mohebi, "Advanced scalar intensity measures for collapse capacity prediction of steel moment resisting frames with fluid viscous dampers," *Soil Dynamics and Earthquake Engineering*, vol. 109, pp. 102-118, 2018.
- [8] H. R. Jamshidiha and M. Yakhchalian, "New vector-valued intensity measure for predicting the collapse capacity of steel moment resisting frames with viscous dampers," *Soil Dynamics and Earthquake Engineering*, vol. 125, 105625, 2019.
- [9] J. W. Baker and C. A. Cornell, "Vector-valued intensity measures for pulse-like near-fault ground motions," *Engineering Structures*, vol. 30, no. 4, pp. 1048-1057, 2008.
- [10] E. Bojárquez, I. Iervolino, A. Reyes-Salazar, and S. E. Ruiz, "Comparing vector-valued intensity measures for fragility analysis of steel frames in the case of narrow-band ground motions," *Engineering Structures*, vol. 45, pp. 472-480, 2012.
- [11] B. Mohebi, F. Kazemi, and M. Yakhchalian, "Investigating the P-Delta effects on the seismic collapse capacity of adjacent structures," *16th European Conference on Earthquake Engineering (16ECEE)*, 18-21, June, Thessaloniki, Greece, 2018.
- [12] M. Yakhchalian, N. Asgarkhani, and M. Yakhchalian, "Evaluation of deflection amplification factor for steel buckling restrained braced frames," *Journal of Building Engineering*, vol. 30, p. 101228, 2020.
- [13] F. Kazemi and R. Jankowski, "Enhancing seismic performance of rigid and semi-rigid connections equipped with SMA bolts incorporating nonlinear soil-structure interaction," *Engineering Structures*, 2023.
- [14] F. Kazemi and R. Jankowski, "Machine learning-based prediction of seismic limit-state capacity of steel moment-resisting frames considering soil-structure interaction," *Computers & Structures*, p. 274, 106886, 2023.
- [15] N. Asgarkhani, M. Yakhchalian, and B. Mohebi, "Evaluation of approximate methods for estimating residual drift demands in BRBFs," *Engineering Structures*, vol. 224, p. 110849, 2020.
- [16] M. Yakhchalian, M. Yakhchalian, N. Asgarkhani, "An advanced intensity measure for residual drift assessment of steel BRB frames," *Bulletin of Earthquake Engineering*, vol. 19, no. 4, pp. 1931-1955, 2021.
- [17] SAC Joint Venture., "Proceedings of the invitational workshop on steel seismic issues," Report No. SAC 94-01, Los Angeles, CA, 1994.
- [18] SAC Joint Venture., "State of the art report on systems performance of steel moment resisting frames subject to earthquake ground shaking," Report No. FEMA 355C, 2000.
- [19] Kazemi, F., Mohebi, B., Jankowski, R., "Predicting the seismic collapse capacity of adjacent SMRFs retrofitted with fluid viscous dampers in pounding condition," *Mechanical Systems and Signal Processing*, 161, 107939, 2021.
- [20] F. Kazemi, M. Miari, R. Jankowski, "Investigating the effects of structural pounding on the seismic performance of adjacent RC and steel MRFs," *Bulletin of Earthquake Engineering*, vol. 19, no. 1, pp. 317-343, 2021.
- [21] American Society of Civil Engineers, "Minimum Design Loads for Buildings and Other Structures (ASCE/SEI 7-10)," American Society of Civil Engineers, 2013.
- [22] F. Kazemi, B. Mohebi, and M. Yakhchalian, "Predicting the seismic collapse capacity of adjacent structures prone to pounding," *Canadian Journal of Civil Engineering*, pp. 663-677, 2020.
- [23] F. Kazemi, N. Asgarkhani, and R. Jankowski, "Predicting seismic response of SMRFs founded on different soil types using machine learning techniques," *Engineering Structures*, 114953, 2023.
- [24] N. Asgarkhani, F. Kazemi, and R. Jankowski, "Optimal retrofit strategy using viscous dampers between adjacent RC and SMRFs prone to earthquake-induced pounding," *Archives of Civil and Mechanical Engineering*, 23(1), 1-26, 2023.
- [25] Kazemi, F., Asgarkhani, N., Jankowski, R. "Probabilistic assessment of SMRFs with infill masonry walls incorporating nonlinear soil-structure interaction," *Bulletin of Earthquake Engineering*, 2023.
- [26] F. McKenna, G. L. Fenves, M. H. Scott, "Open system for earthquake engineering simulation," *University of California, Berkeley, CA*, 2000.
- [27] F. Kazemi, N. Asgarkhani, A. Manguri, and R. Jankowski, "Investigating an optimal computational strategy to retrofit buildings with implementing viscous dampers," *International Conference on Computational Science*, Springer, Cham, 2022, pp. 184-191.
- [28] A. Yahyazadeh and M. Yakhchalian, "Probabilistic residual drift assessment of SMRFs with linear and nonlinear viscous dampers," *Journal of Constructional Steel Research*, vol. 148, pp. 409-421, 2018.

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